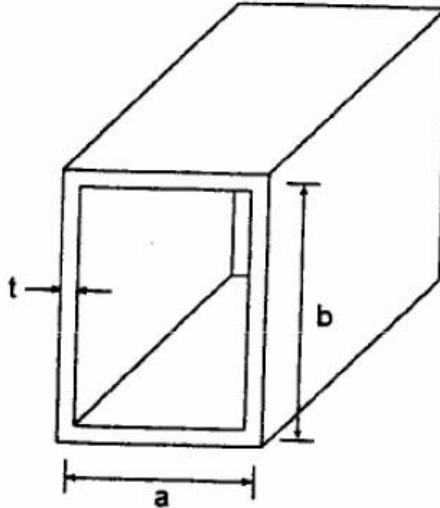


- 1.1** The beam of a rectangular thin-walled section (i.e.,  $t$  is very small) is designed to carry both bending moment  $M$  and torque  $T$ . If the total wall contour length  $L = 2(a + b)$  (see Fig. 1.16) is fixed, find the optimum  $b/a$  ratio to achieve the most efficient section if  $M = T$  and  $\sigma_{allowable} = 2\tau_{allowable}$ . Note that for closed thin-walled sections such as the one in Fig.1.16, the shear stress due to torsion is

$$\tau = \frac{T}{2abt}$$



**Figure 1.16** Closed thin-walled section

**Solution:**

- (1) The bending stress of beams is  $\sigma = \frac{My}{I}$ , where  $y$  is the distance from the neutral axis. The moment of inertia  $I$  of the cross-section can be calculated by considering the four segments of thin walls and using the formula for a rectangular section with height  $h$  and width  $w$ .  $I = \sum (\frac{1}{12}wh^3 + Ad^2)$  in which  $A$  is the cross-sectional area of the segment and  $d$  is the distance of the centroid of the segment to the neutral axis. Note that the Parallel Axis Theorem is applied. The result is  $I = 2 \cdot \frac{1}{12}tb^3 + 2 \cdot [\frac{1}{12} \cdot at^3 + (at) \cdot (\frac{b}{2})^2] \approx \frac{tb^2}{6}(3a + b)$ , assuming that  $t$  is very small.

- (2) The shear stress due to torsion for a closed thin-walled section shown above is

$$\tau = \frac{T}{2abt}.$$

(3) Two approaches are employed to find the solution.

- (i) Assume that the bending stress reaches the allowable  $\sigma_{allowable}$  first and find the corresponding bending maximum bending moment. Then apply the stated loading condition of  $T = M$  to check whether the corresponding  $\tau_{max}$  has exceeded the allowable shear stress  $\tau_{allowable}$ . If this condition is violated, then the optimized b/a ratio is not valid.

$$(a) \quad \sigma \Big|_{y=\frac{b}{2}} = \frac{My}{I} = \frac{M \cdot \frac{b}{2}}{\frac{tb^2}{6}(3a+b)} = \frac{3M}{tb(3a+b)}$$

When given  $L = 2(a+b)$  as a constant,  $a$  can be expressed in terms of  $b$

and  $L$  as  $a = \frac{L}{2} - b$ . Then we can minimize

$$S = \frac{tb(3a+b)}{3} = \frac{tb(3L-4b)}{6} \quad \text{in order to maximize } \sigma, \text{ i.e.,}$$

$$\frac{\partial S}{\partial b} = 0 \Rightarrow \frac{t}{6}(3L-8b) = 0 \Rightarrow b = \frac{3L}{8}, \text{ so } a = \frac{L}{2} - b = \frac{L}{8}$$

where the optimum ratio is  $\frac{b}{a} = 3$

$$\text{Thus, } \sigma_{max} = \frac{3M}{tb(3a+b)} = \frac{3M}{t \cdot (3L/8) \cdot (3 \cdot L/8 + 3L/8)} = \frac{32M}{3tL^2}$$

- (b) Check  $\tau_{max}$  with  $T = M$  and  $b/a = 3$  and check whether  $\tau_{max}$  is within the allowable shear stress  $\tau_{allowable}$ .

$$\begin{aligned} \tau_{max} &= \frac{T}{2abt} = \frac{M}{2 \cdot (L/8) \cdot (3L/8) \cdot t} = \frac{32M}{3tL^2} = \sigma_{max} = \sigma_{allowable} \\ &> \tau_{allowable} = \frac{\sigma_{allowable}}{2} \end{aligned}$$

The result above means that under this assumption, shear stress  $\tau$  would reach the allowable stress  $\tau_{allowable}$  before  $\sigma$  reaches  $\sigma_{allowable}$ . Consequently, the optimal ratio obtained is not valid and different assumption needs to be made.

- (ii) Assume now that failure is controlled by shear stress. We assume that

$\tau_{max} = \tau_{allowable}$  is reached first and then find the corresponding bending stress according to the loading condition  $M = T$ .

$$(a) \quad \tau = \frac{T}{2abt}$$

Again we minimize  $S = 2abt = (L-2b)bt$  in order to maximize  $\tau$ , i.e.,

$$\frac{\partial S}{\partial b} = 0 \Rightarrow (L - 4b) = 0 \Rightarrow b = \frac{L}{4}, \text{ so } a = \frac{L}{2} - b = \frac{L}{4}$$

and the optimum ratio is  $\frac{b}{a} = 1$

$$\text{and } \tau_{\max} = \frac{T}{2abt} = \frac{T}{2 \cdot (L/4) \cdot (L/4) \cdot t} = \frac{8T}{tL^2}$$

(b) Then corresponding  $\sigma_{\max}$  under the optimum condition stated above can be obtained using  $M = T$ . We have

$$\begin{aligned} \sigma_{\max} &= \frac{3M}{tb(3a+b)} = \frac{3T}{t \cdot (L/4) \cdot (3 \cdot L/4 + L/4)} = \frac{12T}{tL^2} = \frac{3}{2} \tau_{\max} = \frac{3}{2} \tau_{\text{allowable}} \\ &< \sigma_{\text{allowable}} = 2\tau_{\text{allowable}} \end{aligned}$$

This means that when the structure fails in shear, the bending stress is

still within the allowable stress level. Thus the optimum ratio  $\frac{b}{a} = 1$  is

valid.

(4) In conclusion,  $\frac{b}{a} = 1$  achieves the most efficient section for the stated conditions.

--- ANS